MEC-E1005 Abaqus workshop

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Agenda, Day 1

- Overview of Abaqus
- Basic use and concepts
- Geometry modelling
- Assigning materials and properties
- Meshing and elements
- Loads and Boundary conditions
- Submitting analyses
- Viewing results
- Static analysis Workshops
- Vibration mode analysis Workshop



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What is Abaqus?

- Suite of finite element analysis modules
- Graphical user interface Abaqus/CAE
- Solvers Abaqus/Standard & Abaqus/Explicit





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- Graphical user interface to create, edit, monitor, diagnose, and visualize Abaqus analyses
- Supports familiar interactive computer-aided engineering concepts as
 - feature-based, parametric modelling
 - interactive and scripted operation
 - GUI customization
- Provides the most complete interface with the Abaqus solver programs available.
- Uses neutral database files that are machine independent (Windows ⇔ Linux)
- Powerful extensibility & customization
 - Python scripting language
 - Automate repetitive tasks with macros
 - Build customized GUIs



Simulation Let Knowledge be your guide

- Functionality is presented in modules.
- Each module contains a logical subset of the overall functionality.
- Once you understand the presentation of one module, you can easily understand the presentation of the other modules.





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- Model Tree and the Results Tree
 - The Model Tree provides you with a graphical overview of your model and the objects that it contains.
 - The Results Tree is used to display analysis results from output databases as well as session-specific data such as X-Y plots.
 - Both trees provide shortcuts to much of the functionality of the main menu bar, the module toolboxes, and the various managers.





Starting Abaqus/CAE

- 1. Command line
 - abaqus cae
 - Opens Abaqus/CAE in current directory
- 2 Windows Start menu

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- Opens Abaqus/CAE in startup directory set during installation
- Double-click . cae or . odb file in Windows folder 3.
 - Opens Abaqus/CAE in current directory

Suggested workflow when starting new project

- Start Abaqus/CAE from Start menu
- Create and/or select a new directory for the project ٠
- Now all analysis and results files are created in this ٠ directory, and things stay organized
 - Otherwise, the default work directory given during installation is used for all files







Option 2

Basic use of Abaqus/CAE

- View manipulation
 - Toolbar to control view (pan, zoom, rotate, etc.).
 - Alternatively, can use a combination of keyboard and mouse actions:
 - Rotate: [Ctrl]+[Alt]+MB1.
 - Pan: [Ctrl]+[Alt]+MB2.
 - Zoom: [Ctrl]+[Alt]+MB3.
 - You can reconfigure these combinations to mimic the view manipulation interfaces used by other common CAD applications
- Selecting entities from main window
 - Add to selection: [Shift] + MB1
 - Remove from selection: [Ctrl] + MB1



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			HyperView	
	\sim	Zoo	Pro/ENGINEER Wildfire	
			UGS NX	
				11





- What is a model database file (extension .cae)?
 - Contains all the information for any number of models.
 - Typically contains one model or several related models.
 - Only one model database can be opened in Abaqus/CAE at a time.



What is a model?

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- Contains all the necessary information for an analysis.
- Contains any number of parts and their associated properties.
- Is independent of other models in the model database.
 - Objects such as parts and materials can be copied between different models in the same database.
- Contains a single assembly of part instances, including the associated contact interactions, loads and boundary conditions, mesh, and analysis history.



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Basic use of Abaqus/CAE

- Models can be imported into one database from another
- Model data from the imported database is copied into the current database.
 - E.g., parts, sections, assemblies, materials, loads, BCs, etc.
- Analysis job definitions and custom data are not copied



	🔜 Import Model from Model Datab		
File Model Viewport View	Model Database: D:/users/ttf/testbed/c		
New Model Database Open Network ODB Connector Close ODB	Model to import: bottle Keep original model name New name:		
Set <u>W</u> ork Directory	After import, show "Model->Copy Ob		
<u>S</u> ave Save <u>A</u> s Compress M <u>D</u> B Sa <u>v</u> e Display Options	OK		
Import	▶ Part		
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Macro Manager			
Print	Ctrl+P		
A <u>b</u> agus PDE			



- Abaqus/Standard analysis types
 - Static stress/displacement analysis:
 - Linear and nonlinear analysis
 - Rate-dependent or rate-independent response
 - Eigenvalue buckling load prediction
 - Linear dynamics:
 - Natural frequency extraction
 - Modal superposition
 - Harmonic loading
 - Response spectrum analysis
 - Random loading
 - Linear/Nonlinear dynamics:
 - Transient dynamics, Implicit time integration



Articulation of an automotive boot seal





- Other analysis types available in Abaqus/Standard:
 - Heat transfer
 - Acoustics
 - Mass diffusion
 - Steady-state transport
 - Multiphysics with Abaqus/Standard:
 - Thermal-mechanical analysis
 - Structural-acoustic analysis
 - Thermal-electrical (Joule heating) analysis
 - Linear piezoelectric analysis



Steady-state transport: rolling of a tire on a drum





Thermal stresses in an exhaust manifold



- Abaqus/Explicit
 - Simulates high speed dynamic events such as drop tests.
 - Explicit algorithm for updating the mechanical response.
 - Also, a powerful tool for highly nonlinear quasi-static analyses
 - Material plasticity & damage, multiple compilated contacts, large deformations,...
 - Annealing is available for multistep forming simulations





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Rolling of a symmetric I-section



Drop test of a cell phone

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- Multiphysics with Abaqus/Explicit
 - Thermal-mechanical analysis
 - Fully coupled: Explicit algorithms for both the mechanical and thermal responses
 - Can include adiabatic heating effects
 - Structural-acoustic analysis
 - Fluid-structure interaction





Two-stage forging, using ALE—contours of temperature



- Special features of Abaqus/Explicit: ALE
 - Adaptive meshing using ALE techniques allows the robust solution of highly nonlinear problems.
 - Mesh adaptivity is based on solution variables as well as minimum element distortion.
 - Elements concentrate in areas where they are needed.
 - Adaptation is based on boundary curvature.
- Special features of Abaqus/Explicit: Coupled Eulerian-Lagrangian (CEL)
 - Define a domain in which material can flow for an Eulerian analysis
 - Flow problems

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- Structural problems with extreme deformation





Bulk metal forming

High speed impact



Abagus uses no inherent set of units.

ka/m³.

- User inputs numbers and Abagus works with these
- It is the user's responsibility to use consistent units. Examples: ٠

mJ(10-3J)

tonne/mm³

Common systems of consistent units



psi (lbf/in²)

in lbf

lbf s2 /in*

ft lbf

slua/ft³



Energy

Density

- For **boundary conditions** and **point loads**, the default coordinate system is the rectangular Cartesian system.
 - Alternative local rectangular, cylindrical, and spherical systems can be defined.
 - These local directions **do not** rotate with the material in large-displacement analyses.





- For material directions (i.e., directions associated with an element's material or integration points) the default coordinate system depends on the element type:
 - Solid elements use global rectangular Cartesian system.
 - Shell and membrane elements use a projection of the global Cartesian system onto the surface.



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Default material directions for solid elements



membrane elements

- Alternative rectangular, cylindrical, and spherical coordinate systems may be defined. _
 - Affects input: anisotropic material directions. _
 - Affects output: stress/strain output directions. _
 - Local material directions rotate with the material in large-displacement analyses. _



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Post processing

- Degrees of freedom
 - Primary solution variables at the nodes.
 - Available nodal degrees of freedom depend on the element type.
 - Each degree of freedom is labeled with a number: 1=x-displacement, 2=y-displacement, 11=temperature, etc.





Demonstration of Abaqus/CAE basic functionality



Static analysis of I-profile cantilever beam

- IPE80 - h = 80 - b = 46 - t = 5.2- s = 3.8
- E = 206 GPa, v = 0.3
- F = 1000 N
- L = 2000 mm
- Use beam elements





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Step 1: Create a part

For beam elements

- Modelled geometry: Line/Wire
- Data given by section properties:
 - Material

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- Profile shape
- Profile Orientation



Step 2: Create material, profile, and beam section



Step 3: Assign beam orientation



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Step 4: Create the Assembly

- An "Instance" of a part is created in the assembly
- Multiple instances of the same part can be created ٠

Assembly Display Options...

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- Part instances can be moved and rotated as needed _
- If changes are made to the part; modified dimensions, materials, section properties and assignments,... all instances of the part in the assembly are updated
- **IMPORTANT:** The assembly is what is used in the analysis
 - Parts that are not instanced are not considered in the analysis



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6

Assembly module

Model: 📮 Model-1

O Models

Select

beam part

📥 Create Instance

Parts

Parts

FAM

Create instances from:

Step

×

Cancel

Module: Assembly

~ ĸ

ť n²

100

li, Ita,

Create

Instance

Step 5: Create analysis step

Analysis step defines what is calculated

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Step 6: Define loads and boundary conditions

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Step 8: Create a Job and submit the analysis



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Step 9: Visualize the results

Check that the job has completed from the Model tree or Job manager

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Visualization module

Tips for result visualization

Rendering beam profiles and shell thickness

DDB C	isplay Options			×		123
General	Entity Display	Constraints	Sweep/Extrude	Mirror/Pattern		2 3 4
Curved	Lines & Faces —				◀┿	
Refinem	ent level: Coars	e 🗸			-	
Elemen	ts with No Result	3				L L,
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Tips for result visualization

- For solid elements visualization is trivial: Result value at each point is unambiguous
- · For shell elements the result value can be from various locations in the thickness of the shell
- · For beam elements the result value can be from various locations of the profile cross section



Example of beam element output. Location of stress visualized by colours is given in the legend. Note that the Mises stress in each cross section is constant because the Lower flange left corner value is plotted for the whole visualized cross section.

Exception: BEAM_STRESS output plots the stress correctly across the beam cross section. However, this output variable is valid only for linear elastic material. Do not use with plasticity.



Result Plot Animate R Step/Frame... Active Steps/Frames... Section Points... Field Output... History Output... Options...

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Location in the shell thickness direction or the position in beam cross section used for output is selected from Main menu > Result > Section Points

Static analysis of U-profile cantilever beam





Step 1: Create a shell part

- This time shell elements are used to model the beam
 - The cross section is modelled as geometry
 - Material and thickness are given as section properties





After sketching, exit the sketcher and give 2000 for the extrusion depth.

💠 Edit Feature 🛛 🗙					
ID: 1					
Name: Shell extrude-1					
Parameters					
Depth: 2000					
Sketches					
Section: 🥖					
Regenerate on OK					
OK Apply C	Cancel				



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Step 2: Create and assign section

Create a shell section

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💠 Create Sectio	on X	🜩 Edit Section	>
Name: Section-1	1	Name: Section-1	
Category Ty O Solid	rpe omogeneous	Type: Shell / Continuum Shell, Homogeneous	
Shell Co Beam Suite	omposite embrane	Section integration: During analysis Before analysis	
O Other Ge	Cancer	Thickness Shell thickness Shell thickness 6 6 6 6 0 Rement distribution: 0 Nodal distribution:	f(x)
		Material: STEEL Material (Creat Thickness integration rule: Simpson Gauss Thickness integration points: 5	ed earlie

OK.



Tip: After Section assignment the shell thickness can be visualized: Main menu > Part Display Options > Render Shell Thickness

Sketch of cross section was done using the middle surface dimensions, so Middle surface is selected. Optionally the thickness can be offset with reference to the modelled geometrical surface

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Cancel
Next steps

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- Similarly as in the first example:
 - Create the assembly
 - Create an analysis step (Static, General)
 - Apply loads and boundary conditions —
 - Mesh the part
 - Create an analysis job
 - Submit the analysis





General and Linear perturbation steps

- Abaqus has General steps and Linear perturbation steps
- Linear perturbation = small displacements around a base state
 - Linear analysis \Rightarrow
 - Small displacements
 - Only elastic material response considered
 - Contact status remains the same as in base state
- Base state:
 - Initial state of model, if linear perturbation is the first step of the analysis
 - State at the end of the preceding General step where NLGEOM is on
- Linear perturbation steps do not effect the time history of a sequence of General steps
 - Duration of linear perturbation step is 1e-36

🐨 Create Step	×
Name: Step-1	
Insert new step after	
Initial	
Procedure type General	
Procedure type General Dynamic, Temp-disp, Explicit	~
Procedure type General Dynamic, Temp-disp, Explicit Geostatic	~
Procedure type General Dynamic, Temp-disp, Explicit Geostatic Heat transfer	^
Procedure type General Dynamic, Temp-disp, Explicit Geostatic Heat transfer Mass diffusion	^
Procedure type General Dynamic, Temp-disp, Explicit Geostatic Heas transfer Mass diffusion Soils	~
Procedure type General Dynamic, Temp-disp, Explicit Geostatic Heat transfer Mass diffusion Soils State, General	^
Procedure type General Dynamic, Temp-disp, Explicit Geostatic Heat transfer Mass diffusion Soils Static, General Static, General Static, Riss	



General and Linear perturbation steps

- Example: Vibration modes of tensioned string
- Sequence of steps



Agenda day 2

- Geometric nonlinearity
- Material nonlinearity
- Constraints and contacts
- Nonlinear static analysis Workshops
- Eigenmode buckling analysis Workshop
- Dynamic analysis Workshop



Linear and non-linear problems

- Properties of the linear problem
 - Load scaling
 - Calculated result U (can be displacement, stress,...) due to load F
 - If load F is increased 4x, the result U will be also 4x bigger
 - Load superposition
 - Load F_1 causes result U_1
 - Load F_2 causes result U_2
 - If loads F_1 and F_2 act at the same time, the result $U = U_1 + U_2$
 - The final state is independent of the loading order.
 - If the load F_1 is applied first and then the load F_2 , the result U will be the same as in the case when first load F_2 and then load F_1 would be applied.
- If the problem in non-linear the above-mentioned statements do not hold true.



Linear and non-linear problems

- The most common causes of non-linearity in structural analysis:
 - Large displacements; geometric non-linearity
 - Non-linear material behavior; material non-linearity
 - Plasticity
 - Hyperelasticity
 - Material damage model
 - Contact
 - Opening and closing of contact
 - Change in the friction contact state: slip/stick
 - Non-linear connectors
 - Non-linear springs
 - Locking/opening/damage connectors



Some examples of geometric non-linearity:

- Effect on load carrying mechanism
 - Shell begins to carry load in the membrane mode
 - Directions of support forces change (rope)
- Effect on loading
 - Change of load orientation or length of moment arm
 - crank mechanism
 - Surface area of pressure load changes
 - inflating a balloon



When must geometric nonlinearity be considered?

- **Good rule of thumb**: If the deformation is not visible by naked eye, there is likely no need to consider geometric nonlinearity.
- Some cases cannot be calculated without considering the geometric nonlinearity.
 - For example, a rope loaded in a transverse direction.



F

Free body diagram

F

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- Example: rope, which has no bending stiffness
 - Carries only tension force in the longitudinal direction
 - Rope force N

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Ν

The equilibrium is found only, when the rope force N gets the vertical component at the support points due to deformation.

F

In the initial state, when the rope is straight, there is no solution for the equilibrium problem.

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Ν

- Steel plate loaded on one side by evenly distributed pressure
 - dimensions: 600 mm x 400 mm
 - thickness: 1 mm

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– material: E = 200 GPa



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Ζ

Loading 100 Pa

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Displacement [mm]



Loading 100 Pa



Stress [MPa]

Loading 25850 Pa = 0.2585 bar



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• Question: What causes this big difference?

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- Answer: As the displacement increases, the plate begins to carry in the membrane mode.
 - SF1 = membrane force in plate [N/mm] in direction x



In the linear model, the membrane force is not generated. It is generated only from the geometrical non-linearity.



Computing time

- In a linear problem the system of equations is solved only once
- Geometrically non-linear analysis requires iteration
 - The system of equations is solved in each iteration
 - Thus, each iteration cycle corresponds to roughly one linear analysis
 - Geometrically non-linear analysis computing time $\sim 2 100$ times the linear analysis
 - If there are other non-linearities, e.g., contact, yielding material, the counting time will increase even more
 - Iterations for material model equations and contact forces



Example of iteration history of the plate analysis involving only geometric nonlinearity



THE ANALYSIS HAS COMPLETED SUCCESSFULLY

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Material nonlinearity

- Plastic material (more details later)
 - When the yield limit is exceeded, permanent deformations arise
 - Scope of the linear analysis: yield limit is not exceeded
- Large strains
 - Elastic strains are so large that the linear material model is not realistic
 - Elastomers, plastic materials, rubber
 - Scope of the linear analysis: maximum strain ca. 5%







Figure 1. Influence of temperature on the behavior of poly(ethylene terephthalate) (PET) under equal biaxial (EB) deformation at a nominal strain rate of 1/s.



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Material nonlinearity

Damage models

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- The yield limit and the elastic stiffness change
 - E.g., concrete, figures below
- Damage cannot be modeled by linear models

Reinforced concrete structure

- Reinforced elastic-plastic model
- Elastic-plastic concrete damage model





Plasticity

- When the load is high enough, permanent deformation begins to occur in the material
 - The deformation will not disappear even if the load is removed
- For metals von Mises plasticity (J2-theory) is most commonly used
- Use in Abaqus:
 - The program is provided with a stress-strain relation, obtained from a tensile test
 - Abaqus calculates the stresses, and based on them, the equivalent von Mises stresses
 - If the yield limit is exceeded, the program iterates until the stress is obtained at the yield limit
 - Iteration: Searching for a deformation state at which the stress is at most at the yield limit and the structure in balance
 - Iterations always increase the computational cost
 - From the calculation effort point of view, each iteration cycle corresponds to approximately one static linear analysis



Plasticity, hardening

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• Hardening: Yield limit grows with plastic deformation



Plasticity, hardening

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Plastic material, strain rate dependence

- The plastic behavior of structural steel depends on the loading rate *) ٠
 - The yield and ultimate strength increase as the load speed increases
- Can be added to the model, rate dependent plasticity
 - The material parameters are given as a function of the strain

0 1

Rate

Plactic

Strain



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FIG. 2.--Strain Rate Effects on Lower Yield Strength of Steels with Different Yield Strengths



FIG. 4.-Strain Rate Effects on Ultimate Strength of Steels with Different Yield Strengths

Plasticity, usage recommendations

- If the load is monotonous (steadily increasing) or the load direction does not change, then
 - At simplest, give only one yield limit σ_{m0}
 - At the simplest, give only one ideal elastic-plastic material, no hardening
 - For numerical reasons, it is generally advisable to use a small hardening
 - Use isotropic hardening, yield limit is given as a function of plastic strain
 - These are easy to calibrate and to implement into the model, computationally light
- When the load is changing (tension-compression) use:
 - Either kinematic
 - Or combined isotropic+kinematic hardening
 - These are more difficult to calibrate, computationally heavy



• Cantilever beam, material S355 (yield limit 355 MPa, small hardening)





- Calculated with the second-order solid tetra elements (Abaqus C3D10)
- Von Mises [MPa] in the figure.
- Question: Is there something strange?





- **Question**: Why is the stress higher than the yield limit?
- **Answer**: The yielding due to bending begins on the surface, however, the material model is evaluated at the integration points inside the element. The stress at the integration points has maximum value of 355 MPa, but when extrapolated, the stress on the surface becomes too large.





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• When calculated with linear shell elements S4R and solid tetrahedrons C3D10HS (integration points on the surface)



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• The largest plastic equivalent strain: shell 0.53%, solid 0.59%



However, even with ordinary solid tetras, the tension result is not as bad as it first appeared



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• The displacement result is practically the same in all models.





Number of DOF of the model

- Shell S4R: 630 (computationally by far the lightest)
- Solid model C3D10: 7446
- Solid model C3D10HS: 7840 (computationally the heaviest)

Question 1: Why does the shell element give the stress result correctly



Answer 1: The shell element has an integration points on the surface where the greatest bending stress is obtained. The shell elements are very good in describing yielding in bending. By default, Abaqus uses 5 integration points in the thickness direction of the shell.

Question 2: What should be considered, when using traditional solid elements in analyses involving plasticity? **Answer 2**: The mesh must be file enough close to the surface to get integration points close to the surface. If being cautious, the higher stresses than yield limit can be accepted. The color scale should be changed in the pictures.

Question 3: Why do C3D10HS elements have more DOF than the C3D10, even though both models have the same number of elements?

Answer 3: The C3D10HS elements incorporate the internal pressure DOF in the calculation. H = Hybrid element



Workshop; Contacting beams

- Use a shell element model
- Dimensions [mm], L x W x t = 400 x 20 x 1
- E = 200 000 MPa, v = 0.3

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- Pressure load on top beam: 0.001 MPa
- Interaction properties for contact





Steps

- Create a shell part. Two options for base feature
 - Planar shell
 - Extruded shell
- Create material and shell section
- Assign shell section
- Mesh part
- Create two instances of the same part in the Assembly
 - Both beams are identical, no need to define two identical parts. Instead, two instances of the same part are created in the assembly
- Position the two beams using the *Translate Instance* tool





Steps, continued

Let **Knowledge** be your quide

Create an analysis step: Static, General

🜩 Edit Step	× 🖶 Edit Step
lame: Step-1 ype: Static, General Basic Incrementation Other Description: Time period: 1 Nigeom: Off Off Off State displacements and affects subsequent steps.) Automatic stabilization: None	Name: Step-1 Type: Static General Basid Incrementation Other Type: Outomatic O Fixed Maximum number of increments 100 Increment size: 0.1 1E-05 1
□ Include adiabatic heating effects Switch on Nonlinear geometry, as large displacements are expected	Reduce the initial time increment to 0.1
OK Cancel	OK Cancel

In static analysis time does not have a physical meaning. It is used to increment the loads and describe the order in which thing happen.

The time period of the step is set to 1. The initial increment is set to 0.1. This means, that Abaqus will try to apply 10% of the load and find an equilibrium state for the structure. After this Abaqus increases the load and again finds an equilibrium state. This process is repeated until the full load has been applied.

The incrementation of the load is automatically adjusted by Abaqus based on the convergence history.

- In the Interaction module create a Contact property and Contact interaction as shown on the previous slide
 - By default parts in the assembly do not interact with each other in any way
 - The analyst must define contacts or constraints between the parts

Steps, continued

- Create an analysis Job and submit the analysis
- Check the results

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Displacements of contacting beams



	💠 Job	Manager			×	1						
	Name		Model		Trne		Status	Write Innut				
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	monitored							Results Rall				
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	1	42	2	,	3	5	0.725519	0.725515	0.0243824			
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	- 1- 4	44	1	2	3	5	0.820848	0.820848	0.0365736			
maller increm	enτ	45	1	2	2	4	0.857422	0.857422	0.0365736			
	1	46	1U	5	2	7	0.857422	0.857422	0.0548604			
	1	46	2	0	3	3	0.871137	0.871137	0.0137151			
	1	47	1	0	3	3	0.89171	0.89171	0.0205727			
	1	48	1	2	2	4	0.922569	0.922569	0.030859			
	1	49	1	2	2	4	0.968857	0.968857	0.0462885			
1 Log Subn Sart Com Sean Text t	1	50	1	2	2	4	1		0.031143	~		
	Log F	rrors ! Warr	inas Ou	tout Data	File Mes	sage File	Status File					
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	Kill Dismiss											

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Material plasticity

- Plasticity data are entered using true stress and logarithmic plastic strain.
- If necessary, convert nominal strain to log strain using the equation (1) given on the right
- When a material has incompressible behavior (as is the case for rubber and most metals when they deform plastically), the relationship between "true" stress and nominal stress is given by equation (2) on the right
- Use the following relation to determine the log plastic strain: $\varepsilon^{pl} = \varepsilon - \varepsilon^{el} = \varepsilon - \sigma/E.$





Material plasticity

• Adding plasticity to Contacting beams example

	Manager 1		
arrie: escripti	ion:		
Materi	al Behaviors		
Elastic			
Plastic			
<u>G</u> ener	ral <u>M</u> echanica	<u>Thermal</u> <u>Electrical/Magnetic</u> <u>O</u> ther	✓
Plastic Use Harder Use	e scale stress valu ning: Isotropic e strain-rate-dep	e:	▼ Suboptions
Use	e temperature-d	pendent data	
Numb	er of field variab	es: 0 🛉	
Data			
	Yield Stress	Plastic Strain	
	235	0	
1		0.1	
1 2	250	0.1	

Plasticity added into material definition

Simulation

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RELEASE step added

	Name	BEND	RELEASE			Edit
V	PRES	Created	Deactivated			Move Lef
						Mouro Right
	Pres <i>REL</i>	sure loa EASE st	d deactiva ep	ited in	۱	Move Rigi Activate Deactivat
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- Possible starting points for buckling analysis
 - As first analysis step: Geometry as modelled, Load: Only buckling load
 - As first analysis step with *INITIAL IMPERFECTION: Geometry modified by imperfection, Load: Only buckling load
 - After a general step with NLGEOM=ON: Deformed geometry from previous step, Load: Loads from previous step as "dead loads", Buckling load with respect to loads applied in Buckling step
- Creating a Buckling analysis step

Let **Knowledge** be vour auide

🚔 Create Sten 🛛 🗙	💠 Edit Step 🛛 🗡			
· · · · · · · · · · · · · · · · · · ·	Name: Step-1			
Name: Buckle	Type: Buckle			
nsert new step after	Basic Other Description:			
nitial				
	Nigeom: On 🧳			
	Eigensolver: 💿 Lanczos 🔿 Subspace			
rocedure type: 📕 Linear perturbation 🗸	Number of eigenvalues requested: 10			
Ruckla	Minimum eigenvalue of interest:			
requency	Maximum eigenvalue of interest:			
atic. Linear perturbation	Block size: Default Value:			
Steady-state dynamics, Direct	Maximum number of block Lanczos steps:			
Substructure generation				
Continue Cancel	OK			

Suggested to start with Lanczos eigensolver with default settings

• Example: Buckling analysis as the first step:





• Example with "dead load"

Simulation

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Apply dead loads, Calculate static equilibrium Apply active loads, Calculate buckling



General notes

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- Eigenvalue is with respect to all active loads acting simultaneously
 - Most load types can be used
 - Nonzero boundary conditions can be applied
 - The Lanczos eigensolver cannot be used with
 - a model containing hybrid elements or connector elements
 - distributing coupling constraints
 - contact interactions
 - a model that has been preloaded over the buckling load
 - a model that has rigid body modes



Buckling of a cylindrical shell

- Material. E = 70 000 MPa, v = 0.3
- Height = 520 mm, Radius = 300 mm
- Distance between reinforcement rings = 125 mm
- Thicknesses: Cylinder: 5 mm, Rings: 10 mm



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Dynamic analysis of I-profile cantilever beam

- IPE80 - h = 80 - b = 46 - t = 5.2- s = 3.8
- E = 206 GPa, v = 0.3, $\rho = 7.85e-9$ Ton/mm³
- p = 0.05 MPa
- L = 2000 mm
- Use shell elements

Task 1: Static analysis

- Calculate static deformation for reference
- Task 2: Vibration modes

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 Find frequency of vibration mode that is likely to be excited by dynamic loading

Task 3: Transient dynamic analysis

Calculate transient dynamic response (see next slide)



Assumption of static analysis: Load is applied slowly, vibrations are not excited.

How much higher is the maximum displacement, if the same pressure load is suddenly applied on the beam?

Dynamic analysis of I-profile cantilever beam

- Module: 🖨 Step ⇒ Edit Step Name: Step-1 Type: Dynamic, Implicit 🔷 Edit Step Basic Incrementation Other Procedure type: General Name: Step-1 Type: () Automatic () Fixed Type: Dynamic, Implicit .+. Direct cyclic Maximum number of increments: 1000 Basic Incrementation Other Dynamic, Implicit Initial Minimum Create step Increment size: 0.001 7E-07 Dynamic, Subspace Description: Maximum increment size: O Analysis application default Static, General Time period Specify: 0.001 Static, Riks Nigeom: On Half-increment Residual Visco Suppress calculation Application Transient fidelity Note: May be automatically suppresse ed when application is not set to transient fidelity. Include adiabatic heating effects Analysis product defa O Specify scale factor: Tolerance: O Specify value: Cancel Continue... ОK Cancel οк Cancel Based on the frequency of the eigenmode, set the Set the maximum time increment to duration of the step to approx. 1/10 of the period of the approximately 5 vibration eigenmode. This ensures that enough cycles time points are used and the time
- Step definition for transient dynamic step

Increase the maximum number of time increments to ensure that analysis does not end prematurely

history output is accurate.

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Where to find more information?

- Abaqus Example Problems, Verification, and Benchmarks in documentation
 - Input files and other input data for all examples are available
 - Example:

🔶 > Abaqus > Example Problems > Static Stress/Displacement Analyses >

Static and quasi-static stress analyses > <u>Axisymmetric analysis of bolted pipe flange connections</u>

Input files

boltpipeflange_axi_solidgask.inp

Axisymmetric analysis containing a gasket modeled with solid continuum elements

boltpipeflange_axi_node.inp

Node definitions for boltpipeflange_axi_solidgask.inp and boltpipeflange_axi_gkax8.inp.

boltpipeflange_axi_element.inp

Element definitions for boltpipeflange_axi_solidgask.inp.

boltpipeflange 3d solidgask.ing

https://help.3ds.com/2020/English/DSSIMULIA_Established/SIMACAEEXARefMap/simaexa-cboltpipeflange.htm?contextscope=all&id=f88c0b11fa174c71b902ec0a53859db4

Open and copy/paste to text file or use Abaqus fetch utility:

abaqus fetch job=boltpipeflange axi solidgask.inp



Where to find more information?



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and Applications

> Learn more

Thank you!

